

March 20, 2025

## Sign Support Structures

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# #EndTheStreakTX

End the streak of daily deaths on Texas roadways.

TxDOT.gov #EndTheStreakTX Toolkit





## **TxDOT Ancillary Structure Standards BRG maintains the** *structural aspects* **of:**

Standard Description	Standards	Current Design Spec.	Upcoming Design Spec.
High Mast Illumination Assembly	HMIP, HMID	LTS-3 (1994)	LTS-6
Roadway Illumination Assembly	RIP, RID	LTS-6 (2013)	LRFD-LTS
Overhead Sign Structures	OSB, HOSB, COSS, HCOSS	LTS-3 (1994)	LRFD-LTS
Monotube Sign Structures	MS, MC	LTS-6 (2013)	
Traffic Signal Poles	SP, SMA, DMA, MA, MAC, MAD, TS, LUM, CFA, LMA	LTS-3 (1994)	LRFD-LTS
Wind and Ice Maps	WV & IZ	LTS-3 (1994) & LTS-6 (2013)	LRFD-LTS

https://www.dot.state.tx.us/insdtdot/orgchart/cmd/cserve/standard/toc.htm 10



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#### **COSS Standards**

#### CANTILEVER OVERHEAD SIGN SUPPORT STANDARDS

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61	COSS-Z1-10	4-10	Cantilever Overhead Sign Supports	FDF stds61.dgn
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63	COSS-Z2I-10	4-10	Cantilever Overhead Sign Supports	FDF stds63.dgn
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66A	COSSD	11-07	Cantilever Overhead Sign Support Details	FDF stds66.dgn
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67	COSSF-21	8-21	Cantilever Overhead Sign Support Foundation	<b>FDF</b> cossf-21.dgn
68	COSS-FD	11-07	Foundation Embedment Selection Charts	RDF stds68.dgn



#### **OSB Standards**

OVERHEAD	SIGN BRIDGE STAND	ARDS		41A	OSB-Z4	8-08	Overhead Sign Bridge Details
OVERHEAD	SIGN DRIDGE STAND	ANDS		41B	OSB-Z4	8-08	Overhead Sign Bridge Details
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31	OSB-SE	11-07	Overhead Sign Bridge Selection Examples	43A	OSB-Z4I	11-07	Overhead Sign Bridge Details
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35A	OSB-Z2I	8-08	Overhead Sign Bridge Details	47B	OSBC-SC-Z1	11-07	Overhead Sign Bridge Truss / Single Column
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39A	OSB-Z3I	8-08	Overhead Sign Bridge Details	52	OSB-ED	11-07	Reinforcing
39B	OSB-Z3I	8-08	Overhead Sign Bridge Details	52	OSB-FD-SC	11-07	Foundation Embedment Selection Charts
40A	HOSB-Z3I	11-07	High Level Overhead Sign Bridge Details	54	COSS & OSB-S7-21	8-21	Overhead Sign Bridge Details
40B	HOSB-Z3I	8-08	High Level Overhead Sign Bridge Details	27	5555 G 655 52 21	i	Overhead sign bridge Details



## **Standard Sheets**

- Include:
  - Wind Zone Map (WV & IZ-14)
  - Standard structure (e.g. COSS-Z1)
  - Structure Details (COSSD)
  - Foundation Details (COSSF and COSS-FD)
  - Sign Brackets
    - SMD(2-5)-24 for typical signs
    - DMS-(HZ)-21 or DMS(TM-1)-16 for Dynamic Message Signs (DMS)
  - Elevation View of structure













### **Removal of Sign Walkway and Lighting Brackets**

- SWW(1)-14 | Sign Walkway and Handrail
- SB(SWL-1)-14 | Support Bracket for Signs, Walkways and Lights
  - Relocated relevant support to truss connection details to SMD(2-5)-24.





## **Elevation and Design Criteria**

#### Length of Span ##.## ##.## ##.## ##.## ##.##' ##.## C Column & Drilled Shaft ##.## — C Truss Sian #1 Sian #n Sign #n+1 ##.## # # Tower Pipe -—Roadway High Bottom of Base Plate Point Natural ground or average elevation of surrounding terrain Drilled Shaft-

Sign Structure Design Details								
Structure Type	COSS							
Roadway	CL Alignment							
Station	##+##.##							
Design	Data							
Applicable Standard	HCOSSZ1, COSS Z1 THRU Z4							
Span Length	## ft							
Sign Area	##.## sq ft							
Standard Sign Area	##.## sq ft							
Design Wind Height	## ft							
Tower Height	## ft							
Tower Diameter	## ft							
Tower Wall Thickness	## ft							
Foundation	n Design							
Shear	##.## kips							
Torsion	###.## kip-ft							
Moment	###.## kip-ft							
Foundation Top Elev	###.## ft							
Foundation Tip Elev	###.## ft							
Drilled Shaft Diameter	## in							
Soil	Sand or Clay							
Penetrometer Value	Ν							

#### Cantilevered Overhead Sign Support



## **Elevation and Design Criteria**

#### Cantilevered Overhead DMS Sign Support



	Sign Structure D	esign Details				
Structure	Туре	COSS or DMS				
Roadwa	ay .	CL Alignment				
Statio	ົ	##+##.##				
	Design I	Data				
Applicable St	andard	SZ				
Span Len	ath	## ft				
Sian An	ea	##.## sa ft				
Desian Wind	Height	## ft				
	Truss De	tails				
$W \times D = Width$	x Depth	# ft x # ft				
Lenath of Tru	ss Panel	End = # ft. Other = # ft				
HS Bolt Dia	meter	#/# in				
Total # of HS Bolts in 1	ower Connection	#				
	Member	1 # x # x #/#				
Chord	HS Bolts Reald	#				
	Member	1 # y # y #/#				
Dead Load Diagonal	HS Bolts Beald	#				
	Member					
Wind Load Diagonal	HE Bolts Bog'd	#				
	Mombor	# 1 # 4 # 4 # /#				
Dead Load Vertical	Member	L # X # X #/#				
	HS BOILS Key'd	#				
Wind Load Strut	Member	L # X # X #/#				
Tours Days	HS Bolts Req'a	#				
Truss Dead	Load	## ID/It				
Truss Defie	ection	#.# in				
	Tower D	etails				
Tower He	ight	## ft				
Tower Diar	neter	## in				
Tower Wall Ti	nickness	## in				
Tower ∆h at 1	russ CL	## in				
Base Plate	Diameter	## in				
	Thickness	## in				
	Circle Diameter	## in				
Anchor Bolt	Number of Bolt	#				
	Bolt Diameter	## in				
	Foundation	Design				
Shear		##.## kips				
Torsio	n	###.## kip-ft				
Momer	nt	###.## kip-ft				
Foundation T	op Elev	###.## ft				
Foundation 1	ip Elev	###.## ft				
Drilled Shaft [	Diameter	## in				
Soil		Sand or Clay				
Penetromete	r Value	N				
Main Shaft	Steel	XX (#XX Bar)				
Shaft Spiral Re	inforcing	#X Spiral @ X in Pitch				
share opirur ne		in opilal granna				

			1			
	Sign Structure D	esign Details		Tower De	etails	
Structure Type COSS or DMS		Tower H	eight	## ft		
Roadwa	y .	CL Alignment	Tower Dia	meter	## in	
Statior	ו	##+##.##	Tower Wall T	hickness	## in	
	Design l	Data	Tower ∆h at	Truss CL	## in	
Applicable St	andard	SZ	Reas Diate	Diameter	## in	
Span Len	gth	## ft	Base Plate	Thickness	## in	
Sign Are	ea	##.## sq ft		Circle Diameter	## in	
Design Wind	Height	## ft	Anchor Bolt	Number of Bolt	#	
	Truss De	etails		Bolt Diameter	## in	
$W \times D = Width$	x Depth	# ft x # ft		Foundation	Design	
Length of Tru	ss Panel	End = # ft, Other = # ft	Shear		##.## kips	
HS Bolt Dia	meter	#/# in	Torsic	on	###.## kip-ft	
Total # of HS Bolts in T	ower Connection	#	Moment		###.## kip-ft	
Chard	Member	L # x # x #/#	Foundation	Top Elev	###.## ft	
Chora	HS Bolts Req'd	#	Foundation	Tip Elev	###.## ft	
Dead Load Diagonal	Member	L # x # x #/#	Drilled Shaft	Diameter	## in	
Deau Luau Diagunai	HS Bolts Req'd	#	Soil		Sand or Clay	
Wind Load Diagonal	Member	L # x # x #/#	Penetromet	er Value	N	
	HS Bolts Req'd	#	Main Shaf	t Steel	XX (#XX Bar)	
Dead Load Vertical Member		L # x # x #/#	Shaft Spiral R	einforcing	#X Spiral @ X in Pitch	
Deau Loau Vertical	HS Bolts Req'd	#				
Wind Load Strut	Member	L # x # x #/#				
Willa Load Strut	HS Bolts Req'd	#				
Truss Dead	Load	## lb/ft			20	
Truss Defle	ction	#.# in				



#### **The Review Process**

The parties involved with the Review Process:





#### **The Review Process –** for COSS and OSB Structures





#### **COSS Standards**

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#### **OSB Standards**

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40B	NUSB-231	6-08	nigh Level Overnead sign bridge Details				

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#### **Span Length**





#### Sign Area







#### **Design Wind Height vs. Column Height**





#### **Cantilever Overhead Sign Structures (COSS)**







#### **Cantilever Overhead Sign Structures (COSS)**

- Given:
  - Cantilever Span = 27.0'
  - Column Height, H = 23.3'
  - Design Wind Height,  $H_d = 27.0'$
  - N = 15
  - Hill County







- Zone 4
- 70 mph
- Design Height, Hd = 27.0'
   ↓
   Standard: COSS-Z4 & Z4I-10



Image: registree			ZO	NE 4	WITH	AND	WIT	(HOU)	T ICE	70	MP	H WI	ND							
Bit Hold		10° SPAN		15' SPAN				-		44/14	20/	SPAN .	_		_		40070070	25' SP48		
The Mark 201 bit 1       The Mark	TOREN PUPE BOLT	BASE TRUES DESIDE LOADS	TOWER POPE BOLT	15 PLATE	19,65	063304	LONDS	104	OR PIPE	80.7	<u> </u>	PLATE	196,05	DESIDE LOS	05 108	ER POPE	80.73	PLATE THAT	IS DESIGN LONDS	
No. 1         No. 1 <th< td=""><td></td><td>CIR SIIX OU'L DEM DEM MONTH</td><td>PA 202 00 80 40</td><td><u>않</u> 1031</td><td>망면</td><td>الم الم</td><td>지수민가</td><td>"eela</td><td>하는 말을 알았는 것</td><td>DIA MG</td><td>쫪</td><td>\$126</td><td>27</td><td>HEN DIDN</td><td>offer hald</td><td>0 C 007</td><td>375 mg</td><td>S 138 2</td><td>Personal primeros</td></th<>		CIR SIIX OU'L DEM DEM MONTH	PA 202 00 80 40	<u>않</u> 1031	망면	الم الم	지수민가	"eela	하는 말을 알았는 것	DIA MG	쫪	\$126	27	HEN DIDN	offer hald	0 C 007	375 mg	S 138 2	Personal primeros	
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1       1	15' 8 8 6,119 8 8	8 8 8 2.74 8 4.2	1 1 0.200 0 I	1 2 - Car 17			43.4	计算能	210 0.481	11	<u> </u>	1	1.1	5.61 8 1	10.14	8 0.342	111	1 1 1	5 7.62 5 115.6	
Norm         Norm <th< td=""><td>16' 0.136</td><td>2,77 40.9</td><td>0, 105</td><td>24% #15</td><td>0.6 4.</td><td>14</td><td>67,6</td><td>3 0.</td><td>250 0.547</td><td></td><td></td><td></td><td>1.8</td><td>1.42</td><td>10.66</td><td>0,435</td><td></td><td></td><td>6 7.08 120.1</td></th<>	16' 0.136	2,77 40.9	0, 105	24% #15	0.6 4.	14	67,6	3 0.	250 0.547				1.8	1.42	10.66	0,435			6 7.08 120.1	
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District       District <th< td=""><td>22' 0.257</td><td>0.2 2.45 60.4</td><td>0.577 4</td><td>45 × 10</td><td>1</td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td>30'</td><td>SPAN</td><td></td><td></td><td></td><td></td><td>1,13 160.3</td></th<>	22' 0.257	0.2 2.45 60.4	0.577 4	45 × 10	1								30'	SPAN					1,13 160.3	
Construct         Construct <t< td=""><td>27' 0.292</td><td>0.2 5.47 45.4</td><td></td><td>1 1 1 1</td><td>85</td><td>· ·</td><td>TOME</td><td></td><td>DE</td><td>1</td><td>ANCH</td><td>IOR</td><td></td><td>BASE</td><td>TOURS</td><td></td><td>CLON</td><td>LOADE</td><td>C 14 134.0</td></t<>	27' 0.292	0.2 5.47 45.4		1 1 1 1	85	· ·	TOME		DE	1	ANCH	IOR		BASE	TOURS		CLON	LOADE	C 14 134.0	
minimum       minim       minimum       minimum	25' 0.331 1 1/4	20 16 24 19 2,44 48,14	9, 745		1 발효	L 1	UNNE	RPI	PE		BOL	TS	- 1	PLATE	INUSS	1 10	510N	LUAUS	1.10 101.1	
Image: State with the state with th	26' 0.250 1 %	20 N (24)(41 N F. 10 12.0	0.006		무별		1.2	4	DEEL	\$17E	_	80	T		DEEL	CHEAD	TODOT	NUMBER	1.20 100.0	
<sup>1</sup> / <sub>1</sub> <td <td="" <td<="" td=""><td>28' 0,416</td><td>2.5 77,6</td><td>0.250 0.935</td><td></td><td>1 -</td><td>0.0.</td><td>너희질</td><td>161</td><td>THE I</td><td>DIA</td><td>Ino.</td><td>l čĩ</td><td>κI</td><td>SIZE</td><td>L V</td><td>1 SHEAR</td><td>l Mar</td><td>MUMERI</td><td>7, 23 202, 0</td></td>	<td>28' 0,416</td> <td>2.5 77,6</td> <td>0.250 0.935</td> <td></td> <td>1 -</td> <td>0.0.</td> <td>너희질</td> <td>161</td> <td>THE I</td> <td>DIA</td> <td>Ino.</td> <td>l čĩ</td> <td>κI</td> <td>SIZE</td> <td>L V</td> <td>1 SHEAR</td> <td>l Mar</td> <td>MUMERI</td> <td>7, 23 202, 0</td>	28' 0,416	2.5 77,6	0.250 0.935		1 -	0.0.	너희질	161	THE I	DIA	Ino.	l čĩ	κI	SIZE	L V	1 SHEAR	l Mar	MUMERI	7, 23 202, 0
No. 1         No. 2         No. 2 <th< td=""><td>29' 0.446</td><td>1 1 2.54 40.5</td><td>0.280 0.498</td><td></td><td>(f+)</td><td>(tin)</td><td>l≨∃</td><td>5 E E</td><td>(in)</td><td>(In)</td><td></td><td>1 ŏî</td><td>ïΙ</td><td>(10)</td><td>1 660</td><td>Kt ine</td><td>dan in</td><td>(K-F+)</td><td>1.24 209.1</td></th<>	29' 0.446	1 1 2.54 40.5	0.280 0.498		(f+)	(tin)	l≨∃	5 E E	(in)	(In)		1 ŏî	ïΙ	(10)	1 660	Kt ine	dan in	(K-F+)	1.24 209.1	
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ZONE 4       WITH AND WITHOUT       15'       4       0.327       4       4       1.6       6.44       114.90         With ADD WITHOUT       10'       0.372       1.7       8.46       149.44         With ADD WITHOUT       10'       0.372       1.7       8.46       149.44         With ADD WITHOUT       10'       0.420       1.8       1.9       0.50       164.45       1.9       1.9       1.5       1.9       1.5       1.9       1.9       1.5       1.9       1.9       1.5       1.9       1.5       1.9       1.5       1.9       1.5       1.9       1.5       1.9       1.5       1.5       1.5<	N2* 16 0.250 0.543 1 12 6	21" 25 x 1) 0.5 2.96 12.39 89.6	1 16 0.280 1.094 1 1/2 8	21 25 × 15	14	24	0.2	250	0.285	1 72	18	23		33 X 1)	2 1.0	8. 42	119.1	01 134.48	1.29 82.44 230.5	
V         V		2015			15'	LŁ.			0.327	-	14	1		- t-	1.6	8.44	1.1	141.90		
No.         No. <td></td> <td>ZUNE</td> <td>WITH AND I</td> <td>WITHOUT</td> <td>16'</td> <td>П</td> <td></td> <td></td> <td>0.372</td> <td></td> <td>П</td> <td>П</td> <td></td> <td></td> <td>1.7</td> <td>8.46</td> <td></td> <td>149.44</td> <td></td>		ZUNE	WITH AND I	WITHOUT	16'	П			0.372		П	П			1.7	8.46		149.44		
No. 10         No. 10<	83 1000 AVA	30' (PAN 28 8455 mars) mars	AND AND	35' SPAN	17'				0.420		++				1.8	8.48		157, 10	and or d	
The state of	The loss first way	S PLATE MADE VESTOR CARDS	1000 P3P4 805	10 15.02	1.01	++	++		0 471	+	++	++	-	_	1.0	0.50	+ +	164.95	sis and infertis	
1       1		CIR SIZE AV STATE	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	206 SLIK	10	++-		-	0.4/1	_	++-		_	_	1.9	0.00	+ +	164.65	1 TO AST# AS3	
11       4       6.22       1       6.22       0       0.581       1       2.2       1       8.54       180.60       100.00 </td <td>14' 24 0.250 0.285 1 1/2 8</td> <td>29" 35 +1% 1.4 4.42118.0[154.4</td> <td>34 0.310 0.406 1 % 8</td> <td>29 % 25 Ka 17</td> <td>19'</td> <td></td> <td></td> <td></td> <td>0.524</td> <td></td> <td>11</td> <td></td> <td></td> <td></td> <td>2.0</td> <td>8.52</td> <td></td> <td>172.68</td> <td>risonor may use</td>	14' 24 0.250 0.285 1 1/2 8	29" 35 +1% 1.4 4.42118.0[154.4	34 0.310 0.406 1 % 8	29 % 25 Ka 17	19'				0.524		11				2.0	8.52		172.68	risonor may use	
Image: Note of the set o	15' 4 6.327 4 4	1.4 8.44 110.8	4 4 0.447 4 8		20'	П	П		0.581	-	П			-	2.1	8.54		180.60	The sone digners	
Image: Second	17 0.420	1.8 8.48 157.1	0.250 0.599	3136117	21/	++	+ +		0 641	1 4	++	20		33 411	6 2 2	0 50		100 50	atesi, corriecti	
1       0.48       1	18' 0,471	1.8 8.55 144.8	0.201 0.402	35Ke+5	21	++-		$\rightarrow$	0.041	1 72	+++	6.0		33 × 17	2 616	0, 50	+ +	100.33	t free and	
1       0.441       1       1       0.441       1       1       0.441       1       1       0.441       1       1       0.441       1       1       0.441       1       1       0.441       1       1       0.441       1       1       0.441       1       1       0.441       1       1       0.441       1       0.441       1       0.441       1       0.441       1       0.441       1       0.441       1       0.441       1       0.441       1       0.441	20' 0.161	2.1 8.54 180.6	0,745		22'	ш.		_	0.703	1 94	ш.	29	%"	33%4×15	2 2.2	8.58		196,65	totes of	
Image: State in the s	211 0.441 1 1/2	29" 35 x1% 2.2 8.56 188.5	0.420	33%a1%	23'				0.768			1		33%×15	2 2.3	8.60		204.76	sving COSSS.	
Image: State in the s	23. 0.249	8 33%x11/g 2.3 8.40 304.3	0.312 0.489 1	1 35 Ball	24'				0.837					33X×11	6 2.4	8.62		212,93	ing between those	
Arr         State         S	24' 0.457	228,419, 2, 4 8, 42 212, 9	4 0.144 1 K	29 % 15 Ke 1 K	05/				0.000	-			-	777	10.5	0.04		001 15	and integer spot.	
Image: State in the intervent interventintervent intervent intervent intervent intervent interv	26 0.250 0.942	23%+152.4 8.44 325.4	1,000 5	to a personal	25	++-			0.908	_	++-		_	3374813	78 2.5	0.04	$\rightarrow$	221.15	<ul> <li>Design Tricitude</li> </ul>	
Image: Construction	27' 0.281 0.949	35Na13 2.4 0.67 231.1	0.312 1.225		26'		0.2	250	0.982		Ш			33%×1	6 2.6	8.66		229.42	sonel and 20 sunds	
No.         No. <td>29' 1,005 1 %</td> <td>29 5 238 13 2.4 8.71 254.4</td> <td>8 1,297</td> <td>340/4 2</td> <td>27'</td> <td>П</td> <td>0.3</td> <td>281</td> <td>0.949</td> <td></td> <td>П</td> <td></td> <td></td> <td>33¾×13</td> <td>4 2.4</td> <td>8.67</td> <td></td> <td>237.74</td> <td>apecified for</td>	29' 1,005 1 %	29 5 238 13 2.4 8.71 254.4	8 1,297	340/4 2	27'	П	0.3	281	0.949		П			33¾×13	4 2.4	8.67		237.74	apecified for	
AV         TRUSS         30'         1.025         2.02         333/4.1 %         2.6         8.71         254.49         production           AV         TRUSS         30'         1.102         2.9 %         33/4.1 %         2.6         8.71         254.49         production         256.2         9.8         7.7         2.7         8.73         256.2         9.8         271.41         2.7         8.73         256.2         9.8         7.7         1.0         2.7         8.73         256.2         9.8         7.7         1.0         2.7         8.73         256.2         9.8         7.7         1.0         1.0         1.2         1.2         9.9         33/2         1.2         2.8         8.75         1.2         1.2         1.2         9.9         1.3         1.2         2.8         8.75         1.2	30' 1.172 2	29 K (14) 1 1 2 7 8 73 242 9	1.577		20'	++	-		1.021	+	Ħ		_	11¥.v13	1. 2.5	0.60		246.10	Winelys,	
29         1.095         1.4         29         33/2         1.1         21         2.6         8.71         254.49         per lease refer           av         TRUSS         30'         1.172         2.1         29         34/2         1.251         2         1.29         4.8         7.5         221.1         1.1         7.7         2.6         8.77         1.90         271.41         7.7         7.7         7.7         2.6         8.77         1.90         271.41         7.7 <t< td=""><td></td><td>or h penalting and he reliated in the</td><td>24 0.340 1.547 2 8</td><td>10 5 2400 2</td><td>20</td><td>++-</td><td>1</td><td>-</td><td>1.021</td><td><del>. l</del>.</td><td>++-</td><td>100</td><td></td><td>33744 17</td><td>4 2.3</td><td>0.03</td><td>++</td><td>240,10</td><td>in brochats,</td></t<>		or h penalting and he reliated in the	24 0.340 1.547 2 8	10 5 2400 2	20	++-	1	-	1.021	<del>. l</del> .	++-	100		33744 17	4 2.3	0.03	++	240,10	in brochats,	
LAV         TRUSS         30'         1.172         2         29         4''         34/2x 1 ½         2.7         8.73         262.93           1         1         1.251         2         9         4''         34/2x 1 ½         2.7         8.73         262.93         271.41           1         1         1         2         1         2         9         4''         34/2x 1 ½         2.8         8.75         271.41         4''         1         1         1         1         2         1         2         9         4'''         34/2x 1 ½         2.8         8.75         271.41         4'''         1         1         1         0         2         2         9         4''''         34/2x 1 ½         2.8         8.77         119.01         279.92         4''''         1         0         1         1         0         1         1<''''''''''''''''''''''''''''''''''''					29,	ц.	$\square$		1.095	1 %	₩.	29	Xe -	3374×17	4 2.6	8.71	$ \rightarrow $	254, 49	ign loads for	
Image: Non-Structure         Image: No	1.v. ***			TRUSS	30'	П			1.172	2	П	29	¥4 *	341/2×13	4 2.7	8.73		262.93		
Construction         Construction<	0000000	Unit of the second seco	10, 10, 6 20	257	31'	11		, _	1.251	2	TŦ	29	Υ.	341/5×13	2.8	8.75		271.41	T	
Notice of Halows-Control         V <td>TRACT</td> <td>CHORD-CL United Otherwise Shown 1.</td> <td>3 × 3 × 5 @ (*)(</td> <td></td> <td>201</td> <td>1 24</td> <td></td> <td>201</td> <td>1 222</td> <td>-</td> <td>1.</td> <td>20</td> <td>V</td> <td>74/</td> <td>1 2 9</td> <td>0 77</td> <td>110</td> <td>1 270 02</td> <td>d Transportatio</td>	TRACT	CHORD-CL United Otherwise Shown 1.	3 × 3 × 5 @ (*)(		201	1 24		201	1 222	-	1.	20	V	74/	1 2 9	0 77	110	1 270 02	d Transportatio	
Image: Section control of the cont		DEAD LOND DEADDWA, - D	2 - 2 - 5 (2) - 2	1213	32	124	10.1	201	1.333	ć	10	129	74	3472813	4 2.0	10.11	11.9.1	01219.92	- Debiter	
United that the first of th		BLAD LOND DEADDHN, - (D) (L)	2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2		14104	1		(2)		\$	1000		-	(2)						
NUM         NUM <td>5</td> <td>FIND LOAD STRUT-OD IL</td> <td>2 . 2 . 5 (1) . 2</td> <td></td> <td>612.5</td> <td>2 + 2</td> <td>- 5</td> <td>(1)</td> <td></td> <td>8</td> <td>(1)</td> <td>2 * *</td> <td></td> <td>(+)</td> <td></td> <td></td> <td>6</td> <td>ANTILEVER</td> <td>R OVERHEAD</td>	5	FIND LOAD STRUT-OD IL	2 . 2 . 5 (1) . 2		612.5	2 + 2	- 5	(1)		8	(1)	2 * *		(+)			6	ANTILEVER	R OVERHEAD	
ELEVATION         No.1 & POINT Processing         4 - N° Bit arr         5 - N° Bit arr         6 -		SIZE IN 5, BOUTS IN CONNECTION	Nº 014	Nº 014	-		014		10 H	004	-		N 10/	4			I '	CTON C	IDDODTC	
ISNOTIONS CONTINUENT OF THE CO	ELEVATION 1	NG & SIZE OF IN 5, BOUTS IN CHORD		4 - %' DIA -	5		- 01A		1.5	OCA or			3.0	A or			1	2104 20	JPPORTS	
LOAD DEFLECTIONS) COSS-24 & 241 COSS-24 & 24	10000000	ENGLE TO TORER CORRECTED PLATE	4 - % 204 00	3 - 34" 014 #	•	5 - 5	- 01A	**	8 - 6-	DCA #4	_	1.	54.0	A #0			1			
LCAD DEFLECTIONS)	LOADS AND DEAD							6	-	Steel"	for a	on-brild	os atr	vertures.				COSS-	Z4 & Z4I-	
Combon Street' for non-doring environments     Per Let up the state of the structures and the structure	LOAD DEFLECTION	5)							be. Ites	en), 'A	etel /	er stru	(huner	e.,			0.00	Contra tor		
								4	Corbon St.	11: 1	r non-	21 Ma	en-uer en-uer	ures.						

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## **Overhead Sign Bridges (OSB)**







## **Overhead Sign Bridges (OSB)**

- Given:
  - Span,  $L_{S} = 93.0'$
  - Left Tower Height,  $H_L = 26.3'$
  - Right Tower Height,  $H_R = 22.6'$
  - Design Wind Height,  $H_d = 27.0'$
  - N = 20
  - Dawson County









- Zone 2
- 90 mph
- Design Height, Hd = 27.0'  $\downarrow$ Standard: OSB-Z2I



The use of hile stronger of is governed by the "texas Engineering Procefloe Apt", No worronty of any kind is made by Tablo of any program strongers. To followers to responsibility for the corres-tation of hile strands to other formats or for Theorem structure.

% Dig. K.S. Bolta	
TRUSS DETAILS	
Speak         40'         45'         50'         50'         60'         65'         70'         75'	
W × D • VETTH × CEPTH         4.5 × 4.5         4.5 × 4.5         4.5 × 4.5         4.5 × 4.5         4.5 × 4.5         4.5 × 4.5	
(2000 - 2), Unless Otherwise Shown L3 x 3 x ½ ③ (3) L3 x 3 x ½ ③ (3) L3 x 3 x ½ ③ (3) L3 x 3 x ½ ④ (3) L3 x 3 x ½ (3) L3	
WIND LOAD STRUT - 10 L2 × 2 × ½ (1) L2 × 2 × 2 × 2 × 2 × 2 × 2 × 2 × 2 × 2	-
TOTAL DEFL. & TRUSS D.L. DEFLAD, 11° L+22 Ib/TH DEFLAD, 22° L+47 Ib/TH DEFLAD, 22° L+47 Ib/TH DEFLAD, 27° L+44 Ib/	
5 - COLIMN SPACING 6.0' 6.0' 6.0' 6.0' 6.0' 6.0' 6.0' 6.0'	NNI .
TOWER HEIGHT	19
$\frac{16'}{16} = \frac{10 \times 15}{10 \times 15} = \frac{10 \times 15}{10 \times 15} = \frac{10 \times 17}{10 \times 15} = \frac{10 \times 17}{10 \times 10} = 10 \times $	JTH × DEPTH
16' 10 x 15 (24.7) 10 x 17 (27.5) 10 x 17 (27.5) 10 x 17 (27.5) 10 x 27 (27.5) 10 x 17 (27.5) 10	Otherwise Shown
mt 19' <sup>↑</sup> 10 × 17 (29,7) × 10 × 22 (36,3) × 10 × 22 (36,3) × 10 × 22 (39,6) × 10	GONAL -(3)
1 20′ 1 1 10 × 17 (31.4) 1 22 (34.3) × 10 × 22 (34.3) × 10 × 22 (41.8) × 10 1 23 × 3 × 1/4 [2] L 3 × 3 × 1/4 [3] L 3 × 3 × 1/4 [3] WIND LOAD DI	GONAL -3
$\frac{1}{27} + \frac{1}{27} + \frac{1}{10} + \frac{1}{10} + \frac{1}{22} + \frac{1}{26} + \frac{1}{10} + \frac{1}{10} + \frac{1}{22} + \frac{1}{26} + \frac{1}{10} + \frac{1}{10} + \frac{1}{26} + \frac{1}{10} + \frac{1}{10} + \frac{1}{26} + \frac{1}{10} $	TICAL -O
語 24 日 1 1 1 2 1/2 x 2 1/2 x 3/6 [1] L 2 1/2 x 3/2 [1] L 2 1/2 [1	itrut - 3
	TDUSS D I
	X TRO35 D.L.
27' 8 10 x 26 (45.6) 12 x 26 (46.4) 12 x 26 (66.4) 12 x 26 (55.1) 12 x 26 (55.4) 12 x 26 (56.4)	
50' # 12 x 26 (49, 3) # 12 x 26 (54, 5) # 12 x 26 (60, 4) # 14 x 30 (67, 1) # 14 7 7.0' 7.0' 7.0' 7.0' S = COLUMN	SPACING
	GHT
W 10 X 26 (37.1) W 10 X 26 (39.3) W 10 X 26 (41.5) W 10 X 26 (43.7) 15'	
ZONE 2 WITH ICE 90 M.P.H. WIND W 10 x 26 (39.7) W 10 x 26 (42.0) W 10 x 26 (44.4) W 10 x 26 (46.8) 16'	
Yr bic K.S. bits         TRUSS DETAILS         W 10 x 26 (42.3)         W 10 x 26 (44.8)         W 12 x 26 (47.6)         W 12 x 26 (50.1)         17'	
apoint to into 100	
45x45 45x45 45x45 45x45 45x45 WxD+WIDFN COPTN	
$\frac{1.3 \frac{1}{3} \times 3 \frac{1}{3} \times \frac{3}{4} \times \frac{3}{4} \times \frac{3}{4} \times \frac{101}{2} \frac{1}{3} \frac{1}{4} \times \frac{3}{4} \times \frac{101}{2} \frac{1}{10} 1$	ec
$13 \times 2 \times \frac{1}{3}$ (2) $13 \times$	<u> </u>
L 2 ½× 2 ½× ¾ (1) who load strut - 🕚 while X 20 (35.3) while X 20 (35.4) while X 20 (39.5) while X 20	뢰
DEFL-0.94" L-14 Hortf DEFL-1.08" L-10 ID/TT	11
TOWER DETAILS	
	§ē
w 14 x 30 (b1.5) W 14 x 30 (b5.9) W 14 x 30 (b5.9) W 14 x 30 (c5.9) W 14 x	<u>•</u>
w 10 x 60 (35, 10) w 10 x 26 (42, 10) w 10 x 26 (46, 67) (10) x 26 (46	Ŧ
<b>*</b> 10 × 26 (42,5) <b>*</b> 10 × 26 (44,6) <b>*</b> 12 × 26 (47,6) <b>*</b> 12 × 26 (50,1) 17 <b>W</b> 14 × 30 (68,0) <b>W</b> 14 × 30 (71,9) <b>W</b> 14 × 34 (75,7) <b>W</b> 14 × 34 (79,8) 7 26'	
	<u>i</u>
w 12 x 26 (35,3) w 12 x 26 (36,4) w 12 x 26 (39,5) w 14 x 30 (45,4) 12 x 26 (45,4) 12 x 2	
w 12 x 26 (56.0) w 12 x 26 (59.3) w 14 x 30 (62.5) w 14 x 30 (66.7) w 22 w w 14 x 34 (76.6) w 16 x 36 (82.2) w 16 x 36 (86.6) w 16 x 36 (91.0) 29(	
	·
$\frac{1}{14 \times 30} \frac{1}{16 \times 30} $	
W 14 x 30 (68,0) W 14 x 30 (75,7) W 14 x 34 (75,6) Z (75,7) W 14 x 34 (75,7) W	
w 14 x 34 (70, 9) w 14 x 34 (74, 9) w 14 x 34 (78, 9) w 16 x 36 (14.2) 3 27' sign below the € of the truis.	
W 14 x 54 (173,77) W 14 x 34 (172,9) W 14 x 54 (192,11) W 16 x 36 (197,6) 8 28'	
III 1 × 20         U(IN_2)         III N × 20         U(IN_2)	
© varent datar motorea ner (* 14m (42) * 14m (14m)	
For Structures,	



varranty of any f for the conver- fing from its use.	9'-0' TO \$\$\$421145 FV\$1 \$594.5135 0'-0' TO \$\$\$54.51 \$500.515 00.001.E ANGLES \$\$\$14' \$\$\$14' \$\$14' \$\$14' \$\$14' \$\$14' \$\$14' \$\$16' \$500.515' \$\$ 00.001.E ANGLES \$\$\$14' \$\$	7'-0'         TO         FOR COLIMM         SPACTNOL         6'-0'         TO         SPAC 6'-0'         TO         6'-0'         TO <th>CING FOR 6° COLUMN SPACING. BIASE BIASE REQUIRED BIASE WT TO W 54 % 74 % 1/2 *</th> <th>ANCHOR BOLT SIZE DIA.x LENGTH DIA.DRILL SHAFT/W REINF.</th> <th>COLLIMN SIZE</th> <th></th>	CING FOR 6° COLUMN SPACING. BIASE BIASE REQUIRED BIASE WT TO W 54 % 74 % 1/2 *	ANCHOR BOLT SIZE DIA.x LENGTH DIA.DRILL SHAFT/W REINF.	COLLIMN SIZE	
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e Texos Engineerin seever 1001 ossum for Incorrect resul	$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	#21 x 68 #21 x 62 #21 x 57 #18 x 55 #18 x 50 #18 x 46 #16 x 40	
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#### **TxDOT Geotechnical Manual - LRFD**

#### Appendix 2

#### **Ancillary Structure Foundations**

When using roadway and traffic standards developed for foundations from TCP information (COSS, High Mast Illumination Poles), use the following correlations (from *Touma and Reese*, 1972) from SPT values acquired in the drilled boring logs:

In Clay: NTCP = 1.5 \* NSPT

In Sand: NTCP = 2.0 \* NSPT

Where, NTCP = equivalent TCP blow counts when using STP information

N<sub>SPT</sub> = uncorrected blow counts from STP in-situ testing

These correlations apply to the standard foundation embedment selection charts regarding TCP information currently referced in the standards.



Cantilever Overh	nead Sign Structures		Overhead Sign Bridges						
Design Parameters:       Notes:       Special       Considerations:	Code: Sign Height: Sign Length: Sign Weight: Sign Coefficient of Drag (Cd): Truss Self Weight: Truss Coefficient of drag (Cd): Governing Wind Velocity & Ice Zones: Alternate Designs of pipe columns are permi requirements shall be per Specification Item The width of a truss member may be adjuste thickness by up to 1/16 inch due to member or other justifiable reasons as indicated in th adjustments are only applicable to standard changes to the SZ Standard must be approve Standard Design does not account for DMS. designed, and a Signed and Sealed SZ provid Minimum baseplate thickness is increased al requirement based on Fatigue research func Spans exceeding 40 feet are not covered on designed in accordance with AASHTO LTS-6 of	LTS-3 (1994) 10 ft 100% of the span length from the centerline of the tower 3 lb/ft <sup>2</sup> 1.2 based on conventional sign Weight of all truss members and gusset plates 2.85 for use with the projected area in the plane of wind loading WV & IZ-14 Itted. The alternate design 650 ed by up to ½ inch and/or the availability, assembly clearances, the shop drawings. These COSS and OSB designs. Any ed by the Engineer of Record. Structures requiring DMS shall be ed in the Contract Plans. bove the Code design led by TxDOT. the standards and must be or LRFD-LTS specifications.	Design Parameters:       Notes:       Special       Considerations:	Code: Sign Height: Sign Length: Sign Length: Sign Coefficient of Drag (Cd): Truss Self Weight: Truss Coefficient of drag (Cd): Truss Coefficient of drag (Cd): Governing Wind Velocity & Ice Zones: Overhead Sign Bridge Towers is not permitte Designs. The width of a truss member may be adjuste thickness by up to 1/16 inch due to member or other justifiable reasons as indicated in the adjustments are only applicable to standard changes to the SZ Standard must be approve Standard Design does not account for DMS. designed, and a Signed and Sealed SZ provid Minimum baseplate thickness is increased al requirement based on Fatigue research fund	LTS-3 (1994) 10 ft 75% of the span length from the centerline of the towers 3 lb/ft <sup>2</sup> 1.2 based on conventional sign Weight of all truss members and gusset plates 2.85 for use with the projected area in the plane of wind loading WV & IZ-14 ed to deviate from the Standard ed by up to ½ inch and/or the availability, assembly clearances, ie shop drawings. These COSS and OSB designs. Any ed by the Engineer of Record. Structures requiring DMS shall be ed.				



## **Double Cantilever**

- Select applicable COSS standard based on the location of the project and Design Wind Height. Note: If Design Wind Height is greater than 30', it is recommended to increase the Wind Zone.
- Determine tower details. Remember to round the total span length (Span A + Span B) up to the nearest tabulated value.
- 3. Determine truss details for Span A and <u>THEN</u> Span B.
- 4. Determine foundation details.
- 5. Determine drilled shaft length.





#### DMS

#### All COSS traffic standards are designed for conventional signs and does not account for a DMS.



- Select applicable COSS standard based on the location of the project and Design Wind Height. Note: It recommended to increase the Wind Zone to account for increased loading from the DMS.
- Determine tower details. Remember to round the total span length (Span A + Span B) up to the nearest tabulated value.
- 3. Determine truss details for Span A and Span B accounting for the full length of both cantilevers as one.
- 4. Determine foundation details.
- 5. Determine drilled shaft length.



#### DMS

- DMS Structures are special designs
  - Typically a higher wind zone is used to account for increased loading
  - e.g. using zone 3 design for a DMS in zone 4
- Drilled shaft diameter based on pipe diameter and bolt circle
- Drilled shaft embedment length based on soil conditions

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#### **OSB - Cantilever**





## **OSB Standards**

OVERHEAD	SIGN BRIDGE STAN	DARDS		41A	OSB-Z4	8-08	Overhead Sign Bridge Details
OVERHEAD	SIGN DRIDGE STAN	DANDJ		41B	OSB-Z4	8-08	Overhead Sign Bridge Details
Page No	Sheet Name	Rev Date	Subject	42A	HOSB-Z4	11-07	High Level Overhead Sign Bridge Details
Fage No.	Sheet Nume	nev bute	Subject	42B	HOSB-Z4	8-08	High Level Overhead Sign Bridge Details
31	OSB-SE	11-07	Overhead Sign Bridge Selection Examples	43A	OSB-Z4I	11-07	Overhead Sign Bridge Details
224	OSB 71	0.00	Overhead Sign Bridge Details	43B	OSB-Z4I	8-08	Overhead Sign Bridge Details
52A	030-21	0-00	Overnead Sign Bridge Details	44A	HOSB-Z4I	8-08	High Level Overhead Sign Bridge Details
32B	OSB-Z1	8-08	Overhead Sign Bridge Details	44B	HOSB-Z4I	8-08	High Level Overhead Sign Bridge Details
33A	HOSB-Z1-21	8-21	High Level Overhead Sign Bridge Details	45A	OSBT-21	8-21	Overhead Sign Bridge Tower Details
33B	HOSB-Z1	8-08	High Level Overhead Sign Bridge Details	45B	OSBT	11-07	Overhead Sign Bridge Tower Details
34A	HOSB-Z1L	8-08	High Level Overhead Sign Bridge Details	46	OSBC	11-07	Overhead Sign Bridge Truss Details
34B	HOSB-Z1L	8-08	High Level Overhead Sign Bridge Details	47A	OSBC-SC-Z1	11-07	Overhead Sign Bridge Truss / Single Column
35A	OSB-Z2I	8-08	Overhead Sign Bridge Details	47B	OSBC-SC-Z1	11-07	Overhead Sign Bridge Truss / Single Column
35B	OSB-Z2I	8-08	Overhead Sign Bridge Details	48A	OSBC-SC-Z2	11-07	Overhead Sign Bridge Truss / Single Column
36A	HOSB-Z2I	8-08	High Level Overhead Sign Bridge Details	48B	OSBC-SC-Z2	11-07	Overhead Sign Bridge Truss / Single Column
36B	HOSB-Z2I	8-08	High Level Overhead Sign Bridge Details	49A	OSBC-SC-Z3	11-07	Overhead Sign Bridge Truss / Single Column
37A	OSB-Z3	11-07	Overhead Sign Bridge Details	49B	OSBC-SC-Z3	11-07	Overhead Sign Bridge Truss / Single Column
37B	OSB-Z3	8-08	Overhead Sign Bridge Details	50A	OSBC-SC-Z4	11-07	Overhead Sign Bridge Truss / Single Column
38A	HOSB-Z3	8-08	High Level Overhead Sign Bridge Details	50B	OSBC-SC-Z4	11-07	Overhead Sign Bridge Truss / Single Column
38B	HOSB-Z3	8-08	High Level Overhead Sign Bridge Details	51	OSBS-SC	11-07	Overhead Sign Bridge Single Column and Drilled Shaft
39A	OSB-Z3I	8-08	Overhead Sign Bridge Details	52	OSB-ED	11-07	Reinforcing
39B	OSB-Z3I	8-08	Overhead Sign Bridge Details	52	036-10	11-07	Foundation Embedment Selection Charts
40A	HOSB-Z3I	11-07	High Level Overhead Sign Bridge Details	54	COSS & OSB-57-21	8-21	Arts
40B	HOSB-Z3I	8-08	High Level Overhead Sign Bridge Details	74	0000 & 000-02-21	0-21	Overnead Sign Bridge Details



#### **Special Zone Sheet**

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•	BASE & SIZE				botton edges of all signs may be placed in line.
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33					③ "Carbon Steel" for non-bridge structures
5	8				per iten 442, "Metal For Structures".
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°.					Detail For bernets
22		OSB	STRUCTURES		5'-0" Panels (Typ) 5'-0" Panels (Typ)
20					Dead Load Diagonal 7 Detail 8 Detail 5
28	STRUCTURE NUL AND STATION		_		
E2	DESIGN WIND HEIGHT, HO LTEETS				
19	LENGTH OF SPAN (feet)				
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					Texas Department of Transportation Standard
	GENERAL NOTES		NOTES ON USAGE		
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	details, shown on standard dra	ings 0581, 058C, COSSD, and COSSF,	COSS cod/or OSB stop	docds are not sufficient to define the COSS	OVERHEAD SIGN
		• • • • • • • • • • • • • • • • • • • •	or 058 design and de	tails.	
	2. Dimensions and connections, she	build be determined, using member si	•		BRIDGE DETAILS
	or combination of members show	on this sheet.	2. These sheets should	not be included in the PS&E package if no	
	hand of plan susant second	and and in an an analysis of the	design data is inclu	ded hereon.	
	a number of high strength boits in indicated in proceets, e.g. (1)	equired in truss connection or spi	te ore	antenna area tala antenna a se antenna a attenna	COSS & OSB-S7-21
			3. If included in the c	ontroct plans this sheet must contain "(MOD)"	
	4. Design of truss includes 3 pour	nds per square foot for sign panel.	20 differ the designatio	and mast be secred by a reads F.E.	TiLE C055-050-52-27.000 (m m (m (m
	pounds per foot for lights, one	50 pounds per foot for wolkway, a	I placed		@1+001 November 2001 Gar Skit Job +15aktr
	as specified for the design sig	n panel.			1-21 *C11/C64
					211/ CDAH* 9417 NJ.

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as Department of Transportation

#### General Notes when using the Special Zone Standard (COSS & OSB-SZ):

- 1. Use tower details, truss details, truss to tower connection, and foundation details, shown on standard drawings OSBT, OSBC, COSSD, and COSSF.
- Dimensions and connections, should be determined, using member size or combination of member size or combination of members shown on this sheet.
- Number of high strength bolts required in truss connection or splice are indicated in brackets, e.g. [3], after the member size.
- Design of truss includes as specified for the Sign Panel (10' H times length of truss)
  - -3 psf for sign panel
  - -20 plf for lights
  - -50 plf for walkway

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1	ANGLE TO TOWER CONNECTION PLATE			3 -	- %	- 1	DIA	ea	1	3	- %	÷.,	DIA	ea	



#### Notes on Usage of the Special Zone Sheet:

- 1. This sheet shall ONLY be included in the PS&E package when the COSS and/or OSB standards are not sufficient to define the COSS or OSB design and details.
- 2. These sheets should not be included in the PS&E package if no design data is included hereon i.e. DO NOT LEAVE BLANK
- 3. If included in the contract plans this, sheet must contain "(MOD)" after the designation and must be sealed by a Texas P.E.





#### Summary

- Every standard for ancillary structures are different
  - Read the general notes on usage
- Current Standards are per LTS-3
- Update coming for LTS-LRFD
- Special Designs per LTS-6 or LTS-LRFD
- Keep an eye out for updates
- Feel free to reach out if you have questions





## **Questions?**

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